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A Nonlinear Potential-Flow Model For Wave-Structure Interaction Using High-Order Finite Differences On Overlapping Grids

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1 Introduction

The potential-flow finite-difference model of [1] applied a σ -transform in the vertical direction in order to solve for fully nonlinear water wave propagation over varying water depths. This model was also applied to wave-structure interaction problems using the *immersed boundary method*, see for example [2]. In this abstract we present a similar finite-difference model for wave-structure interaction problems which abandons the σ -transform and instead applies moving and deformable overset grids. The computational grid in this model is updated according to the nonlinear free-surface motion, and the exact position of the body where the no-flux boundary condition is applied. It is expected that the presented overlapping grid methodology provides more flexibility with respect to grid generation for complicated bodies, especially in three-dimensions (3D). First the accuracy and the convergence of this model is validated using the stream function wave theory. Next the results for the forced motion of a submerged heaving cylinder are compared with the analytical solutions. Note that in this abstract only two-dimensional (2D) results are presented.

2 Formulation

A Cartesian coordinate system is adopted, with the origin at the still water level, and the z-axis pointing vertically upwards. Following the notation of [1], the Laplace equation is written as:

$$\nabla^2 \phi + \phi_{zz} = 0, \quad -h < z < \zeta. \tag{2.1}$$

where the horizontal gradient operator is defined by $\nabla = [\partial/\partial x, \partial/\partial y]$. The instantaneous free-surface position is ζ , and h is the water depth. The kinematic and dynamic free-surface conditions can also be expressed by:

$$\frac{\partial \zeta}{\partial t} = -\nabla \zeta \cdot \nabla \tilde{\phi} + \tilde{w} \left(1 + \nabla \zeta \cdot \nabla \zeta \right), \qquad (2.2)$$

$$\frac{\partial \tilde{\phi}}{\partial t} = -g\zeta - \frac{1}{2}\nabla \tilde{\phi} \cdot \nabla \tilde{\phi} + \frac{1}{2}\tilde{w}^2 \left(1 + \nabla \zeta \cdot \nabla \zeta\right).$$
(2.3)

Here $\tilde{\phi} = \phi(\mathbf{x}, \zeta, t)$ in which $\mathbf{x} = [x, y]$ is a horizontal vector, and $\tilde{w} = \frac{\partial \phi}{\partial z}|_{z=\zeta}$. A no-flux boundary condition is also defined by $\mathbf{n} \cdot \nabla \phi = V_n$ at the solid boundaries of the domain with the normal vectors given by \mathbf{n} and V_n the normal velocity of the boundary.

3 Numerical Method

According to the *Method of Lines*, the spacial and temporal discretization in this model are carried out separately [1]. Using a 4th order finite-difference method, the Laplace equation (2.1) is solved in the computational grid which is comprised of a number of moving and deformable overset grids. An example of the domain is shown in Figure 1. This overset grid has 3 components shown as G_1, G_2 and G_3 . The free-surface grid G_3 is deformable and can be generated using a *Hyperbolic Grid Generation* technique by marching the start curve S_f downwards in the z direction. The component G_2 represents the grid for a circular submerged cylinder. This grid can be moved depending on the desired body boundary condition on S_B at each time step. The background grid G_1 is fixed and only changes in the overlapping regions. The start curve S_f (free-surface) is marched in time using the classical explicit 4th-order Runge-Kutta integration scheme. This is performed by calculation of the solution slopes for elevation ζ and velocity potential $\tilde{\phi}$ from (2.2) and (2.3). The whole overset grid and the overlapping region is updated accordingly at each time step. The velocity potential at the free surface S_f specifies a Dirichlet condition for the Laplace problem. The body boundary condition is also applied as a non-homogeneous Neumann condition on S_B . All spacial discretizations are performed using centered stencils and 2 layers of ghost points are added for each component grid. This computational strategy is implemented inside **Overture** [3].

4 Results and discussions

4.1 Stream function theory

The stream function wave theory of [4] is used in this section to verify the convergence of the presented model. The computational grid for this case is made of only G_3 and G_1 in Figure 1. The initial conditions for ζ and $\tilde{\phi}$ at S_f are specified from the solution to the stream function theory. At both left and right boundaries of the grid, periodic conditions are applied. The computed solution for the surface elevation ζ after 5 wave periods is compared with the analytical results. See Figure 2 and Figure 3. The results are shown for increasing wave steepness in the range $H/\lambda = 0.02, 0.04, \cdots 0.10, 0.11, 0.126$. In all these cases $kh = 2\pi$. Here H is the wave height, λ is the wave length and $k = 2\pi/\lambda$ is the wave number. Note also that a mild filter at each time step is applied on $\tilde{\phi}$ and ζ to ensure numerical stability for the steep wave cases. As can be seen from the figure, the dispersion error increases with the wave steepness.

4.2 Forced oscillation of submerged cylinder

For this case, a submerged circular cylinder with radius a is forced to oscillate in the vertical direction. The submergence depth is $\hat{h} = 3a$. The computational grid is shown in Figure 1. Note that for this grid at the far end of the domain for both G_3 and G_1 a homogeneous Neumann condition is considered. The oscillation amplitude is in the range $\eta/a = 0.2, 0.4, \dots 1.00, 1.25, 1.50, 1.75$. The grid in Figure 1 is part of the solution for the extreme case with $\eta/a = 1.75$. The frequency of the forced motion ω is also specified as $\nu a = 1.0$, where $\nu = \omega^2/g$. The applied force on the body is calculated in time as:

$$F_z = -\rho \int_{S_B} \left(\frac{\partial \phi}{\partial t} + \frac{1}{2} \nabla \phi \cdot \nabla \phi \right) n_z \, dS, \tag{4.1}$$

where ρ is the density of water. A harmonic analysis is then performed to find the amplitudes of the force harmonics. The results are compared with the analytical solution in [5], and are shown in



Figure 1: The deformable and moving overset grid. The grid is for the case where a submerged circular cylinder is performing forced oscillation η in the z direction.



Figure 2: Comparison between the computed free-surface wave elevation ζ with the analytical results from the stream function theory. The results are for increasing steepness, and are obtained after 5 wave periods. The extreme case shown in the bottom right plot is for the wave at 80 % of the breaking limit. For all these cases $kh = 2\pi$.



Figure 3: Convergence studies for the cases shown in Figure 2. The dashed lines in the plots show 1 to 4 slope. Note that N = 1 represents the computational grid with Nx = 11 and $N_z = 14$ number of grid points. The resolution study is performed by $N = [1 \ 1.5 \ 1.7 \ 2 \ 2.5 \ 4]$. The plots in Figure 2 are for the case with N = 4.



Figure 4: Amplitudes of the force harmonics due to the forced motion of the submerged cylinder. The top right plot shows an example of applied force in time which if for the extreme case $\eta/a = 1.75$.

Figure 4. Note also that the analytical solution in [5] is based on the linear free surface conditions. This fact may explain the discrepancies between the solutions for the large motion amplitudes, and we are currently running this case in order to verify or refute this supposition.

5 Conclusions

A new computational strategy bas been presented based on moving and deformable overset grids for the wave-structure interaction problem solved by finite-differences. The accuracy and the convergence of the model has been investigated by comparison to stream function wave theory. The results for the force harmonics due to forced motion of the submerged cylinder are also compared with analytical results, and good agreement has been shown for small amplitude motions.

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